COASTLINE
POWER SOLUTIONS

ENGINEERING STRUCTURAL CALCULATIONS
For
Gillette 152" Frame Gensets

May 5, 2025

152" LG Frame Genset Models:

PR-1800	**SP-3000P**	**T4D-2500**
PR-2400	**SP-3000**	**T4D-3000**
SP-2650	**SP-3500**	**T4D-3500**
SP-2000	**SPVD-5000**	**T4D-6000**
SP-2500P	**SPVD-6000**	

Location: Florida

Designed in compliance with: **2023 Florida Building Code, 8th Edition**
ASCE 7 - 22 Minimum Design Loads for Buildings and Other Structures
2020 Aluminum Association Design Manual
ANSI/AISC 360-22 - Specification for Structural Steel Buildings

Anchoring: 1/2" Bolt/Anchors - Minimum (8) per side (16) total

MATTHEW T. BALDWIN
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NO. 64608
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

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Project Information

Project Name/Model #	- Gillette 152" Frame Gensets
Project Number	-
Project Description	- Sound Attenuated Generator Enclosure
Project Location	- Florida
Customer	-
Mounting Location	- Ground

Enclosure Materials

Roof Beam	- 11 Gauge CRS
Roof Panels	- 0.080 Aluminum Panel - 5052-H34
Wall Panels	- 0.080 Aluminum Panel - 5052-H34
Base Frame/Skid	- Formed Aluminum/Steel 'C' Channel

Components

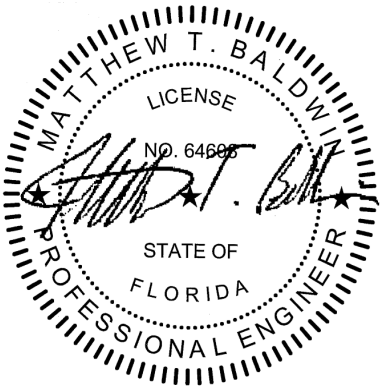
GenSet Manufacturer	- Gillette Supported by - Base
GenSet Size and Model	- PR-1800, PR-2400, SP-2650, SP-2000, SP-2050P, SP-3000P, SP-3000, SP-3500, SPVD-5000, SPVD-6000, T4D-2500, T4D-3000, T4D-3500, T4D-6000
Base	- Formed Aluminum/Steel 'C' Channel

Fasteners/Hardware

		Bolt Size	Washer	Nut	Grade/Finish
Roof to Walls	-	5/16" - 18 Bolts	5/16" Washer	Nut Clip	Grade 18-8/SS
Wall to Wall	-	5/16" - 18 Bolts	5/16" Washer	Nut Clip	Grade 18-8/SS
Walls to Base	-	5/16" - 18 Bolts	5/16" Washer	Nut Clip	Grade 18-8/SS
Base to Slab/Tank	-	1/2" Set Bolt Anchors	Flat Washers	Hex Nuts	Grade 5/Galv.

Specification Requirements

Wind Speed	-	200	mph
Exposure Category	-	D	
Risk Category	-	III	
Ground Snow Load (P_g Fig 7.1)	-	0	psf
Ice Thickness (t Fig 10-2 to10-6)	-	0.25	in
and Concurrent Wind Gust (V_c)	-	30	mph
Seismic Site Class		B	



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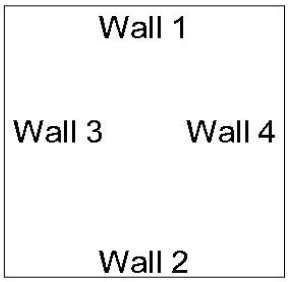
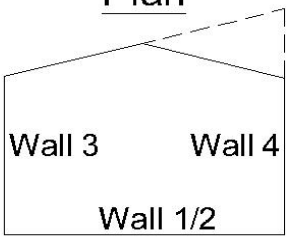
Enclosure Dimensions & Component Weights

Gillette 152" Frame Gensets

Roof Style- Flat



Plan



Elevation

Enclosure Dimensions (ft)

Wall	Length (ft)		Height (ft)
1	6	x	7.21
2	6	x	7.21
3	16.84	x	7.21
4	16.84	x	7.21

Base Dimensions

Width (Wall 1/2 Side)	=	72	in
Length (Wall 3/4 Side)	=	152	in
Height	=	8	in

Roof/Eave Information

Roof Pitch Angle -	*(θ)*	=	0.0	Degrees
Eave/Roof Height -	*h*	=	7.877	

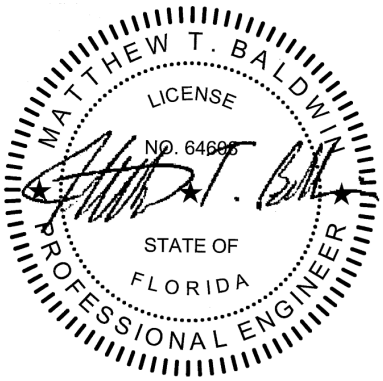
Structure Areas

Walls 1/2 Area -	*(w1)*	=	47.3	ft^2	=	6,805	in^2
Walls 3/4 Area -	*(w3)*	=	132.6	ft^2	=	19,101	in^2
Roof Area -	*(R)*	=	101.0	ft^2	=	14,550	in^2

Base Side 1/2	*(T1)*	=	576.0 in2
Base Side 3/4	*(T3)*	=	1,216.0 in2

Component Weights (lightest setup for worst case)

Genset	=	0	lbs	(Varies, so will use zero to be conservative/most uplift to resist)
Enclosure	=	300	lbs	(Based on Aluminum to be conserative/most uplift to resist)
Base	=	250	lbs	(Based on Aluminum to be conserative/most uplift to resist)



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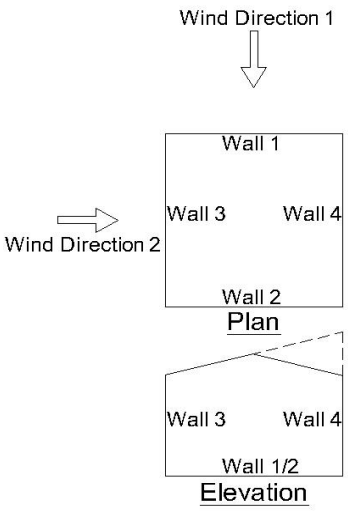
MWFRS Net Pressures

Gillette 152" Frame Gensets

Wind

Analytical Procedure method and Load Combinations from ASCE 7 are utilized in these calculations.

Enclosure Classification	-	Enclosed	
Exposure Category	-	D	
Basic Wind Speed	(V)	200	mph
Importance Factor (Wind)	(I_w)	1.15	
Wind Directionality Factors	(K_d)	0.85	
Internal Pressure Coefficients	(GC_{pi})	± 0.18	
Velocity Pressure Exposure Coefficient	(K_z)	1.03	
Roof Mean Height Above Ground Level	(z)	8.54	ft
Velocity Pressure	(q)	103.12	psf



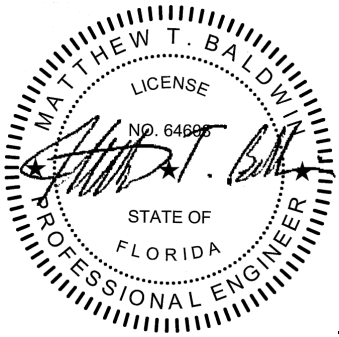
Wind Direction 1

		Enclosure							
		Wall #			Roof				
		1	2	3&4	Parallel to Ridge				
					$(C_p)1$ (Distance From Windward Edge)				$(C_p)2$
		Windward	Leeward	Side	0 to 3.9	3.9 to 7.9	7.9 to 15.8	> 15.8	
Background Response Factor	(Q)	0.97	0.97	0.96	0.97				
Gust Effect Factors	(G)	0.91	0.91	0.90	0.91				
External Pressure Coefficients	(C_p)	0.80	-0.26	-0.70	-0.90	-0.90	-0.50	-0.3	-0.18
Net Pressures with + (GC_{pi}) - psf	(Net_{p+})	56.5	-42.9	-83.8	-103.0	-103.0	-65.5	-46.7	-35.5
Net Pressures with - (GC_{pi}) - psf	(Net_{p-})	93.6	-5.8	-46.7	-65.9	-65.9	-28.4	-9.6	1.7

Wind Direction 2

		Enclosure								
		Wall #			Roof - Normal To Ridge					
		3	4	1&2						
					$(C_p)1$ (Distance From Windward Edge)				$(C_p)2$	
		Windward	Leeward	Side	0 to 3.9	> 3.9				
Background Response Factor	(Q)	0.96	0.96	0.97	0.96					
Gust Effect Factors	(G)	0.90	0.90	0.91	0.90					
External Pressure Coefficients	(C_p)	0.80	-0.5	-0.70	-1.04	-0.70			-0.18	
Net Pressures with + (GC_{pi}) - psf	(Net_{p+})	56.0	-65.2	-84.3	-115.5	-83.8			-35.3	
Net Pressures with - (GC_{pi}) - psf	(Net_{p-})	93.1	-28.0	-47.1	-78.4	-46.7			1.8	

Plus and minus signs signify pressures acting toward or away from the surfaces, respectively.



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Snow

Importance Factor (Snow)	(I_s)	1.1	
Exposure Factor	(C_e)	0.8	
Thermal Factor	(C_t)	1.2	
Slope Factor	(C_s)	1.0	
Flat Roof Snow Load	(p_s)	0	psf

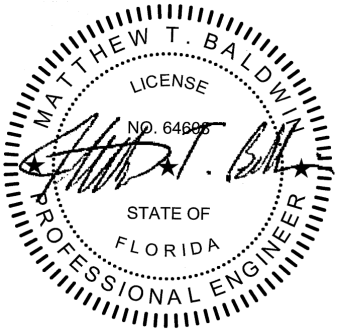
Seismic

Importance Factor (Seismic)	(I_{sm})	1.25		
Mapped Acceleration Parameter	(S_s)	0.14	Figures 22-1 Thru 22-14	
Mapped Acceleration Parameter	(S_1)	0.07	Figures 22-1 Thru 22-14	
Site Coefficient	(F_a)	1		
Site Coefficient	(F_v)	1		
MCE Spectral Resp. Accel. Short Per.	(S_{MS})	0.140		
MCE Spectral Resp. Accel. 1-s Period	(S_{M1})	0.07		
Design Spectral Accel. Short Period	(S_{DS})	0.093		
Design Spectral Accel. 1-s Period	(S_{D1})	0.04667		
Fundamental Period of Structure	(T_a)	0.088	sec	
Long Period Transistion Period	(T_L)	8	sec	Figure 22-15 Thru 22-20
Seismic Design Category	-	A		
Total Effective Seismic Weight	(W_{eff})	2,046	lbs	
Response Modification Coeficient	(R)	2	Table 12.2-1	
System Overstrength Factor	(Ω_o)	2.5	Table 12.2-1	
Deflection Amplification Factor	(C_d)	2	Table 12.2-1	
Seismic Response Coefficient	(C_s)	0.058		

Resultant Seismic Forces

Horizontal Seismic Load Effect	-	(E_h)	
Force at Base of Base	=	0.0	kips
Force at Top of Base	=	0.0	kips
Force at Top/Bottom of Enclosure	=	0.003	kips
Force on Silencer	=	0	kips

Vertical Seismic Load Effect (E_v) = 0 (Factor, Used With Deadweight in Load Combinations)



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Structural Calculations - Roof

Gillette 152" Frame Gensets

Critical Loads & Pressures

Wind Pressures

Downforce	1.784	psf	=	0.01 psi
Uplift	-115.5	psf	=	-0.80 psi

Snow Pressure

0 psf = 0.000 psi

Seismic Load

Horizontal	=	3 lbs
Vertical Factor	=	0

Roof Live Load

Downforce 20.0 psf = 0.1389 psi or 300 lbs Concentrated Load

Pressures & loads are the numerical maximums to be analyzed for shear, bending tension, and compression.

Section Properties

11 Gauge CRS

Cross Sectional Area	(A)	= 0.77	in^2
Moment of Inertia - x	(I_x)	= 0.970	in^4
Moment of Inertia - y	(I_y)	= N/A	in^4
Section Modulus - x	(S_x)	= 0.980	in^3
Section Modulus - y	(S_y)	= N/A	in^3
Radius of Gyration - x	(r_x)	= 1.130	in
Radius of Gyration - y	(r_y)	= N/A	in

Weight	(ω)	= 0.120	lbs/in
Modulus of Elasticity	(E)	= 2.90E+04	ksi
Safety Factor	(Ω)	= 1.95	
Plastic Section Mod. - x	(Z_x)	= 0.24	
Plastic Section Mod. - y	(Z_y)	= 0.24	
Tensile Ultimate Strength	(F_{tu})	= 58	ksi
Tensile Yield Strength	(F_{ty})	= 36	ksi
Compressive Yield Strength	(F_{cy})	= 22	ksi
Shear Ultimate Strength	(F_{su})	= 36	ksi

Roof Frame Calculations

Member Designed for Forces Acting on the Strong Axis

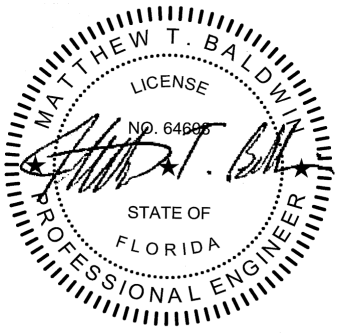
Interior Beam Critical Member Dimensions

Interior Beam Length	(L_i)	= 71.8	in
Load Spanned Width	(W_i)	= 25.35	in

Interior Beam Calculated Forces

Distributed Loads

Weight of Beam	(ω)	= 0.029	lbs/in
Wind Load Downforce	(w_d)	= 0.314	lbs/in
Wind Load Uplift Force	(w_u)	= -20.333	lbs/in



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Shear Forces (Maximum at End)

Beam Weight Shear	(V_b)	=	1.75	lbs
Wind DownForce Shear	(V_{wd})	=	9.7	lbs
Wind Uplift Shear	(V_{wu})	=	-664.3	lbs
Total Shear Downward		=	11.5	lbs
Total Shear Upward		=	662.5	lbs
Design Shear	(V_{bi})	=	662.5	lbs

Stress Forces (Bending)

Beam Weight Moment	(M_b)	=	11	lb·in
Wind Downforce Moment	(M_d)	=	47	lb·in
Wind Uplift Moment	(M_u)	=	-3,223	lb·in
Total Moments Downward		=	59	lb·in
Total Moments Upward		=	3,211	lb·in
Design Moment	(M_T)	=	3,211	lb·in
Design Stress	(σ_{bi})	=	8,921	psi

Interior Beam Design Calculations

Allowable Shear Strength

Slenderness Limit 1	(S_1)	=	-20.08	
Slenderness Limit 2	(S_2)	=	102.40	
Slenderness Ratio	(S)	=	18.0	
Allowable Shear Stress		=	9,856	psi
Allowable Shear Strength	(V_n)	=	3,548	lbs

Conclusion

(V_{bi}) 663 lbs < (V_n) 3,548 lbs **OK**

Allowable Stresses For Tension And Compression (Bending)

Tension

Allowable Tensile Stress (F_t) = 36,960 psi

Compression

Slenderness Limit 1	(S_1)	=	25.0	
Slenderness Limit 2	(S_2)	=	125.0	
Slenderness Ratio	(S)	=	41.3	

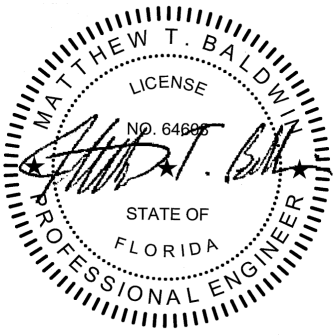
Allowable Compressive Stress (F_c) = 13,121 psi

The Allowable Compressive Stress is the controlling failure design

Therefore, (F_b) = 13,121 psi

Conclusion

(σ_{bi}) 8,921 psi < (F_b) 13,121 psi **OK**



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Entire Roof Uplift Calculations

Roof Area

Area of Roof Subjected to Uplift (R) = 14,550 in^2 (not including discharge hood area)

Roof Uplift Calculated Forces

Roof Weight	(ω_a)	=	102	lbs
Wind Load Uplift Force	(w_{ru})	=	-11,670	lbs
Total Roof Design Uplift	(W_{ru})	=	-11,568	lbs

Mounting Hardware - Roof Frame to Wall Panels

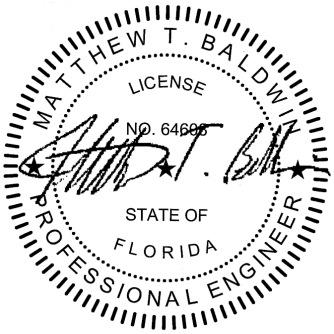
Screws Along Length - 1 Side	=	12	5/16" - 18 Bolts
Screws Along Width - 1 Side	=	4	5/16" - 18 Bolts
Total Mounting Screws	=	32	5/16" - 18 Bolts

Entire Roof Uplift Design Calculations

Grade 18-8/SS Ult. Strength	=	150,000	psi
5/16" Bolt Nominal Diameter	=	0.255	in
5/16" Bolt Effective Area	=	0.051	in^2
5/16" Bolt Threads per Inch	=	18	
Washer Nominal Diameter	=	0.875	in
Wall Panel Tensile Ult. Strength	=	34	ksi
Wall Panel Tensile Yield Strength	=	26	ksi
Safety Factor	=	3	
Wall Panel Nominal Thickness	=	0.0800	in
Maximum Tensile Strength	=	566.7	lbs
Maximum Shear/Bearing Strength	=	408.6	lbs
Max. Tensile Load per Bolt	=	408.6	lbs
Max. Total Screws Tensile Strength (P_{ts})	=	13,074	lbs

Conclusion

(W_{ru}) 11,568 lbs < (P_{ts}) 13,074 lbs **OK**



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Roof Panel Uplift Calculations

Roof Panel Critical Member Dimensions

Critical Panel Length	(L_p)	=	53.40	in
Critical Panel Width	(W_p)	=	72.00	in

Roof Panel Uplift Calculated Forces

Distributed Loads

Wind Load Uplift Force (w_{pu}) = 3,083.8 lbs

Mounting Hardware - Roof Panel to Roof Frame

Screws Along Length - 1 Side	=	4	5/16" - 18 Bolts	- Grade 18-8/SS
Screws Along Width - 1 Side	=	4	5/16" - 18 Bolts	- Grade 18-8/SS

Roof Panel Uplift Design Calculations

Grade 18-8/SS Ult. Strength	=	150,000	psi
5/16" Bolt Nominal Diameter	=	0.255	in
5/16" Bolt Effective Area	=	0.051	in^2
5/16" Bolt Threads per Inch	=	18	
Washer Nominal Diameter	=	0.875	in
Roof Panel Tensile Ult. Strength	=	34	ksi
Roof Panel Tensile Yield Strength	=	26	ksi
Safety Factor	=	3	
Roof Panel Nominal Thickness	=	0.0800	in

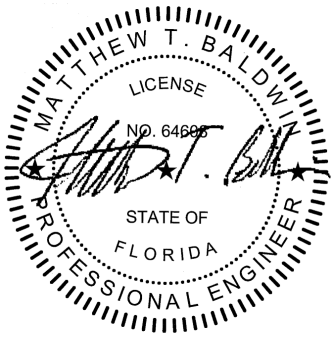
		Roof Frame
Maximum Tensile Strength	=	566.7
Maximum Shear/Bearing Strength	=	408.6
Max. Tensile Load per Screw	=	408.6

(Accounts for screw pull-over and pull-out strengths)

Max. Total Screws Tensile Strength (P_{ts}) = 6,537 lbs

Conclusion

(w_{pu}) 3,084 lbs < (P_{ts}) 6,537 lbs **OK**



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Structural Calculations - Wall Panel

Gillette 152" Frame Gensets

Critical Loads & Pressures

Walls 1 & 2

Maximum Pressures Acting:

Toward	93.6	psf	=	0.6502	psi
Away	-84.3	psf	=	-0.5851	psi

Walls 3 & 4

Maximum Pressures Acting:

Toward	93.1	psf	=	0.6467	psi
Away	-83.8	psf	=	-0.5820	psi

Roof Forces on Critical Panel (From Roof Frame Calculations)

Maximum Downforce	(W_d)	=	2,123	lbs
Wind Load Uplift Force	(w_{pu})	=	3,084	lbs

Pressures and weights are the numerical maximums to be analyzed for shear, tension, and compression.

Critical Wall Panel Dimensions

Critical/Maximum Panel Width	=	68.00	in
Critical/Maximum Panel Height	=	70.00	in

Section Properties

0.080 Aluminum Panel - 5052-H34

Cross Sectional Area	(A)	=	3.79	in^2
Moment of Inertia - x	(I_x)	=	0.050	in^4
Moment of Inertia - y	(I_y)	=	N/A	in^4
Section Modulus - x	(S_x)	=	0.800	in^3
Section Modulus - y	(S_y)	=	N/A	in^3
Radius of Gyration - x	(r_x)	=	0.110	in
Radius of Gyration - y	(r_y)	=	N/a	in
Weight	(ω)	=	0.026	lbs/in^2
Modulus of Elasticity	(E)	=	1.02E+04	ksi
Safety Factor	(Ω)	=	1.95	
Plastic Section Mod. - x	(Z_x)	=	0.13	
Plastic Section Mod. - y	(Z_y)	=	0.13	

Tensile Ultimate Strength	(F_{tu})	=	34	ksi
Tensile Yield Strength	(F_{ty})	=	26	ksi
Compressive Yield Strength	(F_{cy})	=	24	ksi
Shear Ultimate Strength	(F_{su})	=	20	ksi

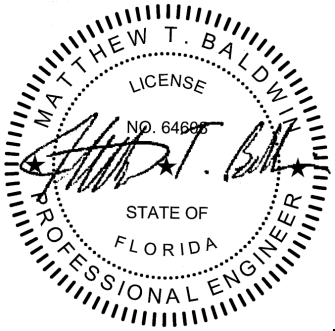
Wall Panel Calculations

Critical Wall Area

Area of Wall Panel	(W)	=	4,760.0	in^2

Mounting Hardware - Roof Frame to Wall Panels

Screws Along Height - 1 Side	=	4	5/16" - 18 Bolts
Screws Along Width - 1 Side	=	8	5/16" - 18 Bolts
Total Mounting Screws	=	24	5/16" - 18 Bolts



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Grade 5 Ultimate Strength	=	150,000	psi
5/16" Bolt Nominal Diameter	=	0.255	in
5/16" Bolt Effective Area	=	0.051	in^2
5/16" Bolt Threads per Inch	=	18	
Washer Nominal Diameter	=	0.875	in
Roof Panel Tensile Ult. Strength	=	34	ksi
Roof Panel Tensile Yield Strength	=	26	ksi
Safety Factor	=	3	
Roof Panel Nominal Thickness	=	0.0800	in

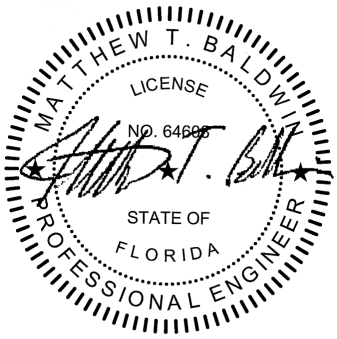
		Roof Frame
Maximum Tensile Strength	=	388.7
Maximum Shear/Bearing Strength	=	300.0
Max. Tensile Load per Bolt	=	300.0

(Accounts for screw pull-over and pull-out strengths)

Max. Total Screws Tensile Strength (P_{ts}) = 6,391 lbs

Conclusion

(w_{pu}) 3,095 lbs < (P_{ts}) 6,391 lbs **OK**



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Structural Calculations - Enclosure to Base

Gillette 152" Frame Gensets

Critical Pressures & Loads

To determine maximum moment forces, pressures are algebraically combined relative to toward or away forces (+ & -) and each wind direction.

Wind Direction 1

To be conservative, roof downforce is neglected.

Net Pressures with + Internal Pressure*(+Gcpi)*

Walls 1 & 2	-	99.4	psf =	0.6906	psi
Wall 3 or 4	-	83.8	psf =	0.5820	psi
Roof Uplift	-	103.0	psf =	0.7154	psi

Net Pressures with - Internal Pressure*(-Gcpi)*

Walls 1 & 2	-	99.4	psf =	0.6906	psi
Wall 3 or 4	-	46.7	psf =	0.3242	psi
Roof Uplift	-	65.9	psf =	0.4576	psi

Wind Direction 2

Net Pressures with + Internal Pressure*(+Gcpi)*

Walls 3 & 4	-	121.2	psf =	0.8415	psi
Wall 1 or 2	-	84.3	psf =	0.5851	psi
Roof Uplift	-	115.5	psf =	0.8021	psi

Net Pressures with - Internal Pressure*(-Gcpi)*

Walls 3 & 4	-	121.2	psf =	0.8415	psi
Wall 1 or 2	-	47.1	psf =	0.3273	psi
Roof Uplift	-	78.4	psf =	0.5443	psi

Seismic

Horizontal Seismic Force (E_h) = 3 lbs

Enclosure Critical Dimensions & Weights

Total Enclosure Weight		(W_t)	=	300.0	lbs	(Includes all components)
Walls 1/2 Area	-	*(w1)*	=	6805.4	in^2	
Walls 3/4 Area	-	*(w3)*	=	19100.6	in^2	
Roof Area	-	*(R)*	=	14549.8	in^2	

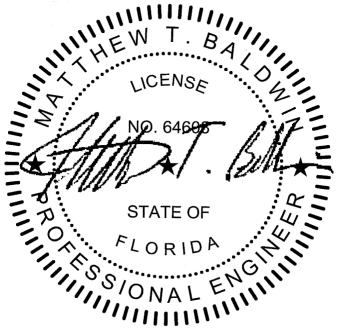
Enclosure Calculated Forces

Maximum Wind Load Forces on Walls

Wind Direction 1

Net Forces with + Internal Pressure*(+Gcpi)*

Walls 1/2	-	=	4,700	lbs
Wall 3 or 4	-	=	11,117	lbs
Roof Uplift	-	=	10,409	lbs



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Net Forces with - Internal Pressure*(-Gcpi)*

Walls 1/2	-	=	4,700	lbs
Wall 3 or 4	-	=	6,192	lbs
Roof Uplift	-	=	6,658	lbs

Wind Direction 2

Net Forces with + Internal Pressure*(+Gcpi)*

Walls 3/4	-	=	16,072	lbs
Wall 1 or 2	-	=	3,982	lbs
Roof Uplift	-	=	11,670	lbs

Net Forces with - Internal Pressure*(-Gcpi)*

Walls 3/4	-	=	16,072	lbs
Wall 1 or 2	-	=	2,227	lbs
Roof Uplift	-	=	7,919	lbs

Enclosure Overturn Forces (Includes Seismic)

(Postive forces act upward, negative forces act downward)

Wind Direction 1

Net Forces with + Internal Pressure*(+Gcpi)*

Overturn on Walls 1/2	=	6,154	lbs
Overturn on Walls 3/4	=	12,351	lbs

Net Forces with - Internal Pressure*(-Gcpi)*

Overturn on Walls 1/2	=	4,278	lbs
Overturn on Walls 3/4	=	7,244	lbs

Wind Direction 2

Net Forces with + Internal Pressure*(+Gcpi)*

Overturn on Walls 3/4	=	16,235	lbs
Overturn on Walls 1/2	=	6,616	lbs

Net Forces with - Internal Pressure*(-Gcpi)*

Overturn on Walls 3/4	=	14,359	lbs
Overturn on Walls 1/2	=	4,330	lbs

Design Overturn Force (O_E) = 16,235 lbs Acting On Wall 3/4

Mounting Hardware - Enclosure to Base/Tank or Pad

To be conservative, bolt connections along the adjacent walls are neglected.

No. of Bolt Connections Along Wall 3/4 = 8 5/16" - 18 Bolts - Grade 18-8/S

Enclosure Overturn Design Calculations

Grade 18-8 Ultimate Strength	=	150,000	psi
Grade 8.8 Nom. Tensile Stress	=	112,500	psi (Includes Reduction Factor)
5/16" Bolt Effective Area	=	0.051	in^2
Tensile Strength per Bolt	=	2,873	lbs

Total Bolts Tensile Strength = 22,982 lbs

Conclusion

(O_E) 16,235 lbs < (R_v) 22,982 lbs **OK**

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Structural Calculations - Enclosure With Base/Tank to Pad

Gillette 152" Frame Gensets

Critical Wind Load Pressures

To determine maximum moment forces, pressures are algebraically combined relative to toward or away forces (+ & -) and each wind direction.

Wind Direction 1

To be conservative, roof downforce is neglected.

Net Pressures with + Internal Pressure *(+Gcpi)*

Walls 1 & 2	-	99.4	psf =	0.6906	psi
Wall 3 or 4	-	83.8	psf =	0.5820	psi
Roof Uplift	-	103.0	psf =	0.7154	psi

Net Pressures with - Internal Pressure *(-Gcpi)*

Walls 1 & 2	-	99.4	psf =	0.6906	psi
Wall 3 or 4	-	46.7	psf =	0.3242	psi
Roof Uplift	-	65.9	psf =	0.4576	psi

Wind Direction 2

Net Pressures with + Internal Pressure *(+Gcpi)*

Walls 3 & 4	-	121.2	psf =	0.8415	psi
Wall 1 or 2	-	84.3	psf =	0.5851	psi
Roof Uplift	-	115.5	psf =	0.8021	psi

Net Pressures with - Internal Pressure *(-Gcpi)*

Walls 3 & 4	-	121.2	psf =	0.8415	psi
Wall 1 or 2	-	47.1	psf =	0.3273	psi
Roof Uplift	-	78.4	psf =	0.5443	psi

Seismic

Enclosure Horiz. Seismic Force	(EE_h)	= 3	lbs
Base/Tank Horiz. Seismic Force	(EB_h)	= 20	lbs

Enclosure With Base/Tank Critical Dimensions & Weights

Total Enclosure Weight		*(W_t)* =	550	lbs	(Includes all components)
Walls 1/2 Area	-	*(w1)* =	7,381	in^2	(Includes Base/Tank Surface Area)
Walls 3/4 Area	-	*(w3)* =	20,317	in^2	(Includes Base/Tank Surface Area)
Roof Area	-	*(R)* =	14,550	in^2	

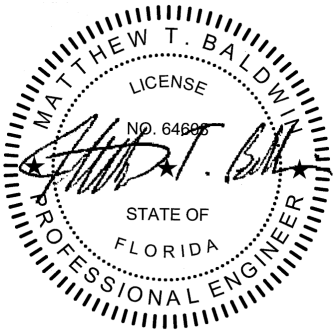
Enclosure With Base/Tank Calculated Forces

Maximum Wind Shear Forces on Walls Including Base/Tank

Wind Direction 1

Net Forces with + Internal Pressure *(+Gcpi)*

Walls 1/2	-	=	5,097	lbs
Wall 3 or 4	-	=	11,824	lbs
Roof Uplift	-	=	10,409	lbs



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Net Forces with - Internal Pressure*(-Gcpi)*

Walls 1/2	-	=	5,097	lbs
Wall 3 or 4	-	=	6,587	lbs
Roof Uplift	-	=	6,658	lbs

Wind Direction 2

Net Forces with + Internal Pressure*(+Gcpi)*

Walls 3/4	-	=	17,096	lbs
Wall 1 or 2	-	=	4,319	lbs
Roof Uplift	-	=	11,670	lbs

Net Forces with - Internal Pressure*(-Gcpi)*

Walls 3/4	-	=	17,096	lbs
Wall 1 or 2	-	=	2,416	lbs
Roof Uplift	-	=	7,919	lbs

Enclosure with Base/Tank Maximum Wind Force = 17,096 lbs Acting On Wall 3/4

Coefficient of Friction - Steel to Wet Concrete (μ_s) = 0.45
Frictional Resisting Force (Total Weight x μ_s) = 248

Enclosure with Base/Tank Design Shear (V_{EB}) = 16,848

Enclosure With Base/Tank Overturn Forces (Inlcudes Seismic)

Postive forces act upward

Wind Direction 1

Net Forces with + Internal Pressure*(+Gcpi)*

Overturn on Walls 1/2	=	6,225	lbs
Overturn on Walls 3/4	=	13,354	lbs

Net Forces with - Internal Pressure*(-Gcpi)*

Overturn on Walls 1/2	=	4,349	lbs
Overturn on Walls 3/4	=	7,750	lbs

Wind Direction 2

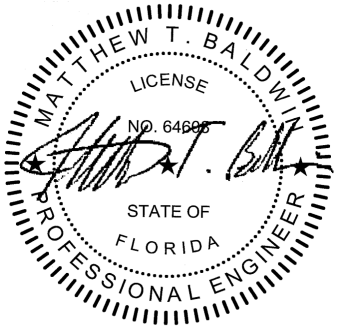
Net Forces with + Internal Pressure*(+Gcpi)*

Overturn on Walls 3/4	=	17,738	lbs
Overturn on Walls 1/2	=	6,658	lbs

Net Forces with - Internal Pressure*(-Gcpi)*

Overturn on Walls 3/4	=	15,862	lbs
Overturn on Walls 1/2	=	4,300	lbs

Design Overturn Force (O_{EB}) = 17,738 lbs Acting On Wall 3/4



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Mounting Hardware - Enclosure With Base/Tank to Pad

No. of Bolt Connections Along Wall 3/4 = 8 Bolts 1/2" Set Bolt Anchors - Grade 5/Galv.

Enclosure With Base/Tank Design Calculations

Mounting Hardware - Shear and Tension

Grade 5	Ultimate Stress	=	120,000	psi
Grade 5	Nom. Shear Stress	=	48,000	psi
Grade 5	Nom. Tensile Stress	=	90,000	psi
1/2" Bolt Nominal Area		=	0.159	in^2
Shear Strength per Bolt		=	3,816	lbs
Tensile Strength per Bolt		=	7,155	lbs
Avail. Tensile Strength per Bolt		=	2,519	lbs (Combined Tension and Shear)

Total Bolts Shear Strength	(R_{vb})	=	30,528	lbs
Total Bolts Tensile Strength	(R_{tb})	=	20,148	lbs

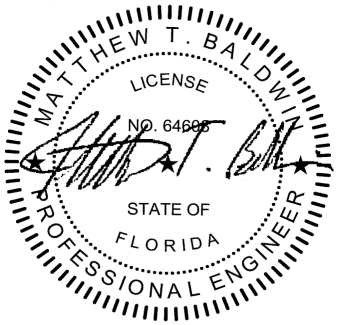
Conclusion

Shear

(V_{EB})	16,848	lbs	<	(R_{tb})	30,528	lbs	**OK**

Tension

(O_{EB})	17,738	lbs	<	(R_{tb})	20,148	lbs	**OK**



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